BOEKENHOUTSKLOOF BONNIEVALE: PROPOSED HOUSING DEVELOPMENT ON ERF 907 & REMAINDER OF FARM 174



TECHNICAL REPORT ON THE PROVISION OF CIVIL SERVICES

AUG 2018

<u>Client:</u> Asla Devco PO Box 116 GORDONS BAY 7150

Consulting Engineers

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V3 CONSULTING ENGINEERS



CONTRACT NO:	10438000
PROJECT DESCRIPTION	V3 Consulting Engineers (Pty) Ltd, was appointed to prepare a services report for the provision of Civil Services to the proposed development of Boekenhoutskloof Housing, Bonnievale.
REPORT DESCRIPTION	Bonnievale: Boekenhoutskloof Housing SERVICES REPORT

Revision	00		
Date	15 Aug 2018		
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Approver's signature

Date

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23 August 2018

Disclaimer:

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1 INTRODUCTION

Asla Devco appointed V3 Consulting Engineers (Pty) Ltd to prepare a services report for the provision of Civil Services to the proposed housing development on Erf 907 & remainder of Farm 174, Bonnievale: Boekenhoutskloof.

The proposed development comprises of the following:

- 438 Residential Zone I erven
- 4 Proposed Government and Municipal Zoned erven
- 3 Open Space erven
- Roads

(Refer to Annexure A for the draft Subdivision Plan prepared by Urban Dynamics Western Cape Town and Regional Planners)

2 DESIGN AND CONSTRUCTION STANDARDS

The minimum standards proposed for the civil services and pertaining infrastructure will comply with the recommendations made by the Department of Human Settlements (Western Cape Government) in respect of the A Grade Engineering Services subsidy.

This is also in compliance with the "Guidelines for Human Settlements, Planning and Design" as compiled under patronage of the Department of Housing by CSIR Building & Construction Technology (also known as the "Red Book") as well as any specific requirements prescribed by the Langeberg Municipality.

The relevant code of standards (i.e. SANS 1200, etc.) should be made applicable to material and construction standards.

3 TOPOGRAPHY

Erf 907 & remainder of Farm 174 (the site) is approximately 11.2 hectare in size and is currently occupied by informal Structures (shacks) and undeveloped grasslands.

The topography of the area is characterized by gentle to moderate sloping terrains (1:6 to 1:16) draining in a south easterly direction into a watercourse to the south and is regarded as suitable for development.

A flood analysis was done by Fraser Consulting Engineers cc and the proposed development is above the 1:100 flood line.

4 GEOTECHNICAL CONDITIONS

No Geotechnical Investigation was available at the stage of the report but no adverse soil conditions are expected in this part of Bonnievale. The site should be suitable for residential development.

A Geotechnical Investigation will need to be undertaken before detail designs can be finalized and proposals made with regard to foundation designs for the streets and the buildings.

5 PROPOSED SERVICES

5.1 Sanitation

Internal Sewage Removal

Sewage will be removed by means of a waterborne gravity sewer network connected to the existing municipal network via a proposed sewer pump station, south east of the development.

The gravity sewer network will consist of 160mm diameter uPVC sewer pipes and 1,0m diameter concrete sewer manholes. The estimated length of the network is 2 580m and approximately 45 manholes will be constructed.

(Refer to attached drawings 10438000-001, -004 and -005 for the preliminary design).

Bulk Sewage Removal

A new sewer pump station with the capacity of 8 ℓ /s will need to be constructed just south east of the development.

The bulk sewage will be removed via a 110mm diameter uPVC rising main. Sewage will be pumped from the proposed new sewer pump station to an existing pump station to the west of the development. The estimated length of the rising main is 2 725m.

The Municipality need to confirm if any upgrades on the existing network, including the existing pump station, will be required.

The additional hydraulic loading on the WWTW system is estimated at 220 kl/day. The Municipality confirmed that the Waste Water Treatment Works (WWTW) have sufficient capacity for the proposed development.

(Refer to attached drawings 10438000-002 and -003 for the preliminary design).

5.2 Water

Internal Water Supply

The development will be serviced by means of a water network consisting of PVC water pipes of 75mm to 110mm diameter (total estimated length 2 618m) installed in road reserves. The proposed connection point is to a proposed 200mm ND UPVC supply line south of the development.

The fire-risk category assumed is "Low-risk (group 4)" for which no specific fire-fighting provision is required. Hydrants will however be located at convenient points ensuring the spacing does not exceed 240m (estimated total = 4).

Isolating valves will be installed in valve chambers and placed in accordance with the design guidelines (estimated total = 12).

(Refer to attached drawings 10438000-006, -008 and -009 for the preliminary design).

Bulk Water Supply for Domestic Usage

The development will be supplied with potable water from the existing Municipal water treatment works by means of a new 200mm ND UPVC pipeline (total estimated length 1 300m).

The estimated water usage is 270 kl/day (@ 600l/erf/day).

The Municipality confirmed adequate water at the recommended residual pressure is available for the development and that the Water Treatment Works (WTW) has sufficient capacity for the development.

(Refer to attached drawings 10438000-007 for the preliminary design).

5.3 Roads and Storm water

Internal Roads

The development will be accessed via an existing gravel road from Bonnievale. All new roads for the proposed development will be provided with a permanent surface finish (25mm Asphalt) and will be 4.5m wide. Edging/kerbs will be provided on the sides of the roads in accordance with the standards. A gravel sidewalk will be provided on one side of the road.

The total estimated area to be surfaced is 13 650m² with approximately 3 000m of kerbs and 3 100m of edging.

It is anticipated that the existing road network leading up to the development will be able to accommodate the increase in traffic. This will however need to be confirmed by means of a TIA of Traffic Statement from a Traffic Engineer.

(Refer to attached drawings 10438000-010, -011 and -012 for the preliminary design).

Storm water drainage and management

Storm water from the proposed development area currently follows preferential drainage paths to the south-east to a larger water course. It is proposed that the storm water be directed in the roads reserves by means of the road geometry, kerbs and storm water pipes through-out the development where it will be discharged in a controlled manner into the existing water course.

To achieve the above, concrete storm water pipes ranging from 375mm to 525mm in diameter (total estimated length = 580m) with associated catch pits and junction boxes will need to be installed.

The southern ravine will need to be crossed to access to the development. An anticipated culvert size of approximately $4 \times 3,0m \times 1,8m$ will need to be installed for the crossing of the ravine to accommodate the 1:100 year flood.

(Refer to attached drawings 10438000-010, -011 and -012 for the preliminary design).

5.4 Refuse Removal

Refuge removal will be done by the Langeberg Municipality as part of their normal services provided.

No hazardous waste is expected to be generated and waste will be normal household waste.

6. <u>CONCLUSION</u>

Pending the confirmation from the Municipality with regard to possible upgrades on the service networks, this development is deemed viable and will contribute positively to the development of Bonnievale.

ANNEXURE A: DRAWINGS

- Draft Layout Plan (Urban Dynamics)
- 10438000-001: Sewer: Layout Plan
- 10438000-002: Bulk Sewer: Plan 1
- 10438000-003: Bulk Sewer: Plan 2
- 10438000-004: Sewer: Detail Plan 1
- 10438000-005: Sewer: Detail Plan 2
- 10438000-006: Water: Layout Plan
- 10438000-007: Bulk Water: Layout Plan
- 10438000-008: Water: Detail Plan 1
- 10438000-009: Water: Detail Plan 2
- 10438000-010: Roads and Storm Water: Layout Plan
- 10438000-011: Roads and Storm Water: Detail Plan1
- 10438000-012: Roads and Storm Water: Detail Plan2













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90	650	400	200
110	650	400	200

NOTES : 1. TO CAST CONCRETE AGAINST UNDISTURBED EXCAVATION "Y" MUST BE INCREASED AS NEEDED 2. BEARING PRESSURE OF SOIL BEHIND THRUST BLOCK MUST BE AT LEAST 40kPa AND THE TEST PRESSURE HEAD IN PIPELINE NOT BIGGER THAN 135kPa 3. TRUST BLOCKS MUST BE BUILT SYMMETRICAL AROUND FITTINGS

BLOCK CAST AGAINST UNDISTURBED EXCAVATION

THRUST BLOCK FOR T-JUNCTIONS

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APPROVED:

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